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**Project no.** 220912 **Date** 24/01/2024

Bojan Ritonja Planit Consulting Pty Ltd PO Box 112 Geelong VIC 3220

Via email bojan@planitconsulting.com.au

Dear Bojan

#### Harvey Road Bannockburn Intersections – Intersection analysis and concept designs

Planit Consulting Pty Ltd engaged Trafficworks to undertake analysis and concept design work for the following intersections along Harvey Road in Bannockburn:

- Harvey Road and Bannockburn-Shelford Road (existing intersection)
- Harvey Road and Ormond Street (existing intersection)
- Harvey Road and new road (does not exist).

The intersections are located within the south west Precinct of the future Bannockburn PSP. Council has identified these intersections will require a contribution from the adjoining landowners and intersection designs are required to inform the contributions required.

The Bannockburn PSP area is shown in Figure 1.







#### **Intersection analysis**

SIDRA analysis was undertaken to determine the indicative intersection layouts required to support the south west precinct of the future Bannockburn PSP. The following assumptions were made to determine the high-level impact of the residential growth area:

- Existing traffic volumes from 2018 were obtained for Harvey Road and Bannockburn-Shelford Road from the Cardno Traffic Modelling report (March 2019).
- The 2018 traffic volumes on Harvey Road and Bannockburn-Shelford Road were projected to the final year (2043) using a 2% compound annual growth rate.
- Ultimately, Bannockburn-Shelford Road will be downgraded to a connector road with the introduction of the bypass arterial road. Therefore, it was assumed that 50% of traffic currently utilising Bannockburn-Shelford Road will use the new bypass arterial road.
- 3% heavy vehicle volumes were applied.



- The lot yield of each area was based on the Bannockburn Growth Plan (May 2021).
- Based on the rates used in the Cardno report, peak hour traffic generation rates of 0.85 trips per dwelling in the AM peak hour and 0.9 trips per dwelling in the PM peak hour were adopted.

The 'south west precinct' includes 51.7ha of business park development. A high-level traffic generation rate of 200 trips / ha of developable land has been applied to estimate the traffic generation for the industrial and business land uses within the PSP. In addition, a ratio of 40% was applied to estimate the equivalent Gross Floor Area (GFA) as a percentage of the total developable land. This rate is equivalent to 5 daily trips / 100m<sup>2</sup> GFA as per the RTA Guide for Traffic Generating Developments.

Based on the above, the anticipated traffic generation of the key areas within the future Bannockburn PSP at full development are shown in Table 1.

Precinct	Measure	Peak hour traff rates	ic generation	Development traffic generation			
		AM Peak	PM Peak	AM Peak	PM Peak		
North west	1,267 lots	0.9	0.85	1,140	1,077		
South west (residential)	1,159 lots	0.9	0.85	1,043	985		
Future growth area - South	2,693 lots	0.9	0.85	2,424	2,289		
South west (business park)	206,721 m²	5 trips / 100 m²	of GFA	1,344			
Total	5,119			5,951	5,695		

Table 1: Traffic generation rates

- The following peak hour splits were applied:

- AM: 20 % IN 80% OUT
- PM: 70% IN 30% OUT

- Traffic distributed to / from the surrounding residential developments would generate:

- 30% to / from Bannockburn town centre
- 60% to / from Geelong
- 10% to west



- Of the local traffic accessing the town centre that generates to the arterial road intersections:
  - 20% will utilise Harvey Road
  - 80% will utilise the arterial roads.
- The 'south east precinct' and 'future growth option east' of the Bannockburn PSP (refer to Figure 1) will not generate traffic to/from the intersections along Harvey Road and therefore have not been included as part of this analysis.

#### Ultimate conditions (i.e. full development)

Based on the above assumptions, the anticipated traffic volumes at full development at the three intersections are shown in Appendix 1.

The intersection layouts required to support the anticipated traffic volumes at each intersection and achieve a 10-year design life are shown in Figures 2 - 4. Detailed SIDRA outputs are provided in Appendix 2.



Figure 2: Harvey Road / Bannockburn-Shelford Road SIDRA intersection layout



**TRAFFICWORKS** 

Figure 3: Harvey Road / Ormond Street SIDRA intersection layout



Figure 4: Harvey Road / future E-W connector road SIDRA intersection layout



#### **Interim conditions**

An interim roundabout treatment is proposed at the Harvey Road / Bannockburn-Shelford Road intersection to facilitate initial development. The proposed roundabout layout is shown in Figure 5.



Figure 5: Harvey Road / Bannockburn-Shelford Road SIDRA interim intersection layout

SIDRA analysis was undertaken to determine the trigger to upgrade the interim intersection layout, based on the following additional assumptions:

- Of the traffic travelling to Geelong:
  - 70% will utilise Bannockburn-Shelford Road
  - 30% will utilise Hamilton Highway via Harvey Road.
- 10% heavy vehicle volumes were applied to Bannockburn-Shelford Road (as per the DTP Open Data Portal).

The SIDRA analysis indicates that the interim roundabout intersection treatment will operate satisfactorily with the development of:

- 1,000 lots from the south west development area only (i.e. no development occurs in the north west development area)
- a combined 1,050 lots from each development area (i.e. south west and north west) assuming the areas develop at the same rate (i.e. 525 lots in the south west and 525 lots in the north west development areas).

#### **Concept designs**



Based on the outcomes of the SIDRA analysis, concept plans were prepared for the interim intersection layouts and are provided in Appendix 3 and swept path analysis is provided in Appendix 4. It is noted that the interim and ultimate layout for the collector road intersections with Harvey Road are the same in both scenarios.

The intersection designs have adopted the following design principles:

- Road cross sections have been based on the standard cross sections prepared by the Victorian Planning Authority (VPA)
- Design speed 60km/h
- Collector roads
  - Design vehicle bus
  - Check vehicle 19.0 m semi-trailer
- Bannockburn-Shelford Road
  - Design vehicle 19.0 m semi-trailer
  - Check vehicle B-double

Please contact me on 0438 343 817 if you require additional information. Yours sincerely,

Ali Abdou **Director** BE (Civil) (Hons), MIEAust, CPEng (Civil, PM), RPEng (Civil)



## Appendix 1 – Anticipated traffic volumes







## Appendix 2 – Detailed SIDRA analysis

#### Site: 101v [Ultimate\_BS Rd-Harvey\_AM - signals (Site Folder: General)]

New Site Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 100 seconds (Site User-Given Cycle Time)

Timings based on settings in the Site Phasing & Timing dialog Phase Times determined by the program Phase Sequence: Convert Function Default Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C Reference Phase: Phase A

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use a	Lane Use and Performance														
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Bac [ Veh	k Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1 Lane 2 Approach	57 391 447	3.0 3.0 3.0	57 391 447	3.0 3.0 3.0	441 484 1	0.129 0.807 0.807	100 100	44.2 52.7 51.6	LOS D LOS D LOS D	2.2 19.3 19.3	15.7 138.9 138.9	Short Full	60 500	0.0 0.0	NA 0.0
East: Bannoo	kburn-Sh	elford F	Road												
Lane 1 Lane 2 Lane 3 Approach	138 169 307 614	3.0 3.0 3.0 3.0	138 169 307 614	3.0 3.0 3.0 3.0	1451 947 947	0.095 0.179 0.324 0.324	100 55 <sup>6</sup> 100	8.1 15.8 16.5 14.4	LOS A LOS B LOS B LOS B	1.5 4.6 9.0 9.0	10.7 32.7 64.4 64.4	Short Short Full	60 100 500	0.0 0.0 0.0	NA NA 0.0
West: Banno	ckburn-Sł	nelford l	Road												
Lane 1 Lane 2 Lane 3 Approach	522 933 21 1477	3.0 3.0 3.0 3.0	522 933 21 1477	3.0 3.0 3.0 3.0	1179 11631 110	0.443 0.803 0.191 0.803	55 6 100 100	399.7 24.5 65.4 157.8	LOS F LOS C LOS E LOS F	13.4 34.5 1.0 34.5	96.4 247.5 7.5 247.5	Short Full Short	100 500 60	0.0 0.0 0.0	NA 0.0 NA
All Vehicles	2538	3.0	2538	3.0		0.807		104.4	LOS F	34.5	247.5				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Reduced capacity due to a short lane effect. Short lane queues may extend into the full-length lanes.

1 Delay and stops experienced by drivers upstream of short lane entry have been accounted for.

6 Lane under-utilisation due to downstream effects

#### **Output Phase Sequence**



Stopped Movement	Turn On Red
Cther Movement Class (MC) Running	Undetected Movement
Mixed Running & Stopped MCs	Continuous Movement
Other Movement Class (MC) Stopped	Phase Transition Applied

#### Phase Timing Summary

Phase	Α	в	С
Phase Change Time (sec)	0	55	88
Green Time (sec)	49	27	6
Phase Time (sec)	55	33	12
Phase Split	55%	33%	12%
Phase Frequency (%)	100.0	100.0	100.0

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

#### Site: 101v [Ultimate\_BS Rd-Harvey\_PM - signals (Site Folder: General)]

New Site Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 100 seconds (Site User-Given Cycle Time)

Timings based on settings in the Site Phasing & Timing dialog Phase Times determined by the program Phase Sequence: Convert Function Default Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C Reference Phase: Phase A

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use a	Lane Use and Performance														
	Demand [ Total veh/h	I Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Back [ Veh	Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1 Lane 2	26 143	3.0 3.0	26 143	3.0 3.0	184 239	0.143 0.599	100 100	54.8 51.7	LOS D LOS D	1.2 6.9	8.7 49.4	Short Full	60 500	0.0 0.0	NA 0.0
Approach	169	3.0	169	3.0		0.599		52.2	LOS D	6.9	49.4				
East: Bannoc	kburn-Sh	elford R	load												
Lane 1	417	3.0	417	3.0	1451	0.287	100	8.6	LOS A	5.4	38.9	Short	60	0.0	NA
Lane 2	419	3.0	419	3.0	1218	0.344	55 <mark>6</mark>	12.6	LOS B	9.5	68.3	Short	100	0.0	NA
Lane 3	760	3.0	760	3.0	1218	0.624	100	12.0	LOS B	22.3	160.1	Full	500	0.0	0.0
Approach	1596	3.0	1596	3.0		0.624		11.3	LOS B	22.3	160.1				
West: Banno	ckburn-Sh	nelford F	Road												
Lane 1	226	3.0	226	3.0	1449	0.156	55 <mark>6</mark>	5.1	LOS A	3.0	21.9	Short	100	0.0	NA
Lane 2	409	3.0	409	3.0	1449	0.282	100	4.2	LOS A	6.2	44.5	Full	500	0.0	0.0
Lane 3	39	3.0	39	3.0	110	0.353	100	57.8	LOS E	2.0	14.1	Short	60	0.0	NA
Approach	674	3.0	674	3.0		0.353		7.6	LOS A	6.2	44.5				
All Vehicles	2439	3.0	2439	3.0		0.624		13.1	LOS B	22.3	160.1				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

6 Lane under-utilisation due to downstream effects

#### Output Phase Sequence



Phase



#### Phase Timing Summary

Phase	Α	в	С
Phase Change Time (sec)	0	69	88
Green Time (sec)	63	13	6
Phase Time (sec)	69	19	12
Phase Split	69%	19%	12%
Phase Frequency (%)	100.0	100.0	100.0

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

#### Site: 101v [Ultimate\_BS Rd-Harvey\_AM - signals - sensitivity (Site Folder: General)]

New Site

Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 150 seconds (Site Practical Cycle Time) Design Life Analysis (Practical Capacity): Results for 10 years

Timings based on settings in the Site Phasing & Timing dialog Phase Times determined by the program Phase Sequence: Convert Function Default Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C Reference Phase: Phase A

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use a	nd Perfo	ormanc	e												
	Demand [ Total	I Flows HV ] %	Arrival [ Total	Flows HV ]	Cap.	Deg. Satn	Lane Util. %	Aver. Delay	Level of Service	95% Back [ Veh	Of Queue Dist ] m	Lane Config	Lane Length	Cap. Adj. %	Prob. Block. %
South: Harve	ev Road	70	VOII/II	/0	VOII/II	10	70					_		/0	70
Lane 1 Lane 2 Approach	57 391 447	3.0 3.0 3.0	57 391 447	3.0 3.0 3.0	429 436 1	0.133 0.896 0.896	100 100	79.8 101.6 98.8	LOS E 11 LOS F 11 LOS F11	3.3 31.3 31.3	23.4 224.4 224.4	Short Full	60 500	0.0 0.0	NA 0.0
East: Banno	ckburn-Sł	nelford l	Road												
Lane 1	138	3.0	138	3.0	1580	0.087	100	7.2	LOS A	1.5	10.6	Short	60	0.0	NA
Lane 2 Lane 3	206 374	3.0 3.0	206 374	3.0 3.0	1134 1134	0.182 0.330	55 <u>6</u> 100	15.9 16.7	LOS B LOS B	6.8 13.7	48.8 98.1	Short Full	100 500	0.0 0.0	NA 0.0
Approach	718	3.0	718	3.0		0.330		14.6	LOS B	13.7	98.1				
West: Banno	ockburn-S	helford	Road												
Lane 1	637	3.0	637	3.0	1288	0.495	55 <mark>6</mark>	2167.4	LOS F 11	22.6	162.4	Short	100	0.0	NA
Lane 2	1137	3.0	1137	3.0	12691	0.896	100	39.6	LOS D	68.9	494.5	Full	500	0.0	<mark>4.0</mark>
Lane 3	21	3.0	21	3.0	73	0.286	100	102.0	LOS F 11	1.6	11.4	Short	60	0.0	NA
Approach	1796	3.0	1796	3.0		0.896		795.7	LOS F <sup>11</sup>	68.9	494.5				
All Vehicles	2961	3.0	2961	3.0		0.896		501.1	LOS F11	68.9	494.5				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

- Reduced capacity due to a short lane effect. Short lane queues may extend into the full-length lanes. 1
- Delay and stops experienced by drivers upstream of short lane entry have been accounted for.
- Lane under-utilisation due to downstream effects 6
- 11 Level of Service is worse than the Level of Service Target specified in the Parameter Settings dialog.

#### Output Phase Sequence Phase A REF Phase B Phase C Shelford Road Shelford Road Bannockburn-Shelford Road Shelford Road Bannockburn Bannockburn Bannockburn זר ור Harvey Road Harvey Road Harvey Road REF: Reference VAR: Variable Phase Normal Movement Permitted/Opposed



#### Phase Timing Summary

Phase	Α	В	С
Phase Change Time (sec)	0	94	138
Green Time (sec)	88	38	6
Phase Time (sec)	94	44	12
Phase Split	63%	29%	8%
Phase Frequency (%)	100.0	100.0	100.0

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

Phase

#### Site: 101v [Ultimate\_BS Rd-Harvey\_PM - signals - sensitivity (Site Folder: General)]

New Site Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 100 seconds (Site User-Given Cycle Time)

Timings based on settings in the Site Phasing & Timing dialog Phase Times determined by the program Phase Sequence: Convert Function Default Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C Reference Phase: Phase A

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use ar	nd Perfo	rmance	9												
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Back [ Veh	Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1 Lane 2	26 143	3.0 3.0	26 143	3.0 3.0	184 239	0.143 0.599	100 100	54.8 51.7	LOS D	1.2 6.9	8.7 49.4	Short Full	60 500	0.0 0.0	NA 0.0
Approach	169	3.0	169	3.0		0.599		52.2	LOS D	6.9	49.4				
East: Bannoc	kburn-Sh	elford R	Road												
Lane 1	417	3.0	417	3.0	1451	0.287	100	8.6	LOS A	5.4	38.9	Short	60	0.0	NA
Lane 2	419	3.0	419	3.0	1218	0.344	55 <mark>6</mark>	12.6	LOS B	9.5	68.3	Short	100	0.0	NA
Lane 3	760	3.0	760	3.0	1218	0.624	100	12.0	LOS B	22.3	160.1	Full	500	0.0	0.0
Approach	1596	3.0	1596	3.0		0.624		11.3	LOS B	22.3	160.1				
West: Bannoo	ckburn-Sh	nelford F	Road												
Lane 1	226	3.0	226	3.0	1449	0.156	55 <mark>6</mark>	5.1	LOS A	3.0	21.9	Short	100	0.0	NA
Lane 2	409	3.0	409	3.0	1449	0.282	100	4.2	LOS A	6.2	44.5	Full	500	0.0	0.0
Lane 3	39	3.0	39	3.0	110	0.353	100	57.8	LOS E	2.0	14.1	Short	60	0.0	NA
Approach	674	3.0	674	3.0		0.353		7.6	LOS A	6.2	44.5				
All Vehicles	2439	3.0	2439	3.0		0.624		13.1	LOS B	22.3	160.1				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

6 Lane under-utilisation due to downstream effects

#### **Output Phase Sequence**



REF: VAR: Variable Phase Reference

Phase



Phase Timing Summary			
Phase	Α	В	С
Phase Change Time (sec)	0	69	88
Green Time (sec)	63	13	6
Phase Time (sec)	69	19	12
Phase Split	69%	19%	12%
Phase Frequency (%)	100.0	100.0	100.0

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

#### VSite: 101 [Ultimate\_Harvey-Connector west\_AM (Site Folder: General)]

New Site Site Category: (None) Give-Way (Two-Way)

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use and Performance															
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Bac [ Veh	k Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1	343	3.0	343	3.0	1912	0.179	100	0.5	LOS A	0.0	0.0	Full	10	0.0	0.0
Approach	343	3.0	343	3.0		0.179		0.5	NA	0.0	0.0				
North: Harvey	/ Road														
Lane 1	179	3.0	179	3.0	1749	0.102	100	1.6	LOS A	0.3	1.9	Full	500	0.0	0.0
Approach	179	3.0	179	3.0		0.102		1.6	NA	0.3	1.9				
West: Connec	ctor Street	(west)													
Lane 1	380	3.0	380	3.0	903	0.421	100	8.7	LOS A	2.4	16.9	Full	500	0.0	0.0
Approach	380	3.0	380	3.0		0.421		8.7	LOS A	2.4	16.9				
All Vehicles	902	3.0	902	3.0		0.421		4.2	NA	2.4	16.9				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

#### VSite: 101 [Ultimate\_Harvey-Connector east\_AM (Site Folder: General)]

New Site Site Category: (None) Give-Way (Two-Way)

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use and Performance															
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Bac [ Veh	k Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harvey	y Road														
Lane 1	301	3.0	301	3.0	1842	0.163	100	0.7	LOS A	0.2	1.5	Full	500	0.0	0.0
Approach	301	3.0	301	3.0		0.163		0.7	NA	0.2	1.5				
East: Connec	tor Street	(east)													
Lane 1	154	3.0	154	3.0	851	0.181	100	8.1	LOS A	0.7	4.8	Full	500	0.0	0.0
Approach	154	3.0	154	3.0		0.181		8.1	LOS A	0.7	4.8				
North: Harvey	/ Road														
Lane 1	394	3.0	394	3.0	1924	0.205	100	0.2	LOS A	0.0	0.0	Full	10	0.0	0.0
Approach	394	3.0	394	3.0		0.205		0.2	NA	0.0	0.0				
All Vehicles	848	3.0	848	3.0		0.205		1.8	NA	0.7	4.8				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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New Site Site Category: (None) Give-Way (Two-Way)

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use a	ane Use and Performance														
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	Aver. Bacl [ Veh	< Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1	343	3.0	343	3.0	1912	0.179	100	0.5	LOS A	0.0	0.0	Full	10	0.0	0.0
Approach	343	3.0	343	3.0		0.179		0.5	NA	0.0	0.0				
North: Harvey	y Road														
Lane 1	179	3.0	179	3.0	1749	0.102	100	1.6	LOS A	0.1	0.8	Full	500	0.0	0.0
Approach	179	3.0	179	3.0		0.102		1.6	NA	0.1	0.8				
West: Conne	ctor Stree	et (west)													
Lane 1	380	3.0	380	3.0	903	0.421	100	8.7	LOS A	0.9	6.8	Full	500	0.0	0.0
Approach	380	3.0	380	3.0		0.421		8.7	LOS A	0.9	6.8				
All Vehicles	902	3.0	902	3.0		0.421		4.2	NA	0.9	6.8				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

## VSite: 101 [Ultimate\_Harvey-Connector east\_AM (Site Folder: General)]

New Site Site Category: (None) Give-Way (Two-Way)

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use a	ine Use and Performance														
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	Aver. Bac [ Veh	k Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1	301	3.0	301	3.0	1842	0.163	100	0.7	LOS A	0.1	0.6	Full	500	0.0	0.0
Approach	301	3.0	301	3.0		0.163		0.7	NA	0.1	0.6				
East: Connec	tor Street	(east)													
Lane 1	154	3.0	154	3.0	851	0.181	100	8.1	LOS A	0.3	1.9	Full	500	0.0	0.0
Approach	154	3.0	154	3.0		0.181		8.1	LOS A	0.3	1.9				
North: Harve	y Road														
Lane 1	394	3.0	394	3.0	1924	0.205	100	0.2	LOS A	0.0	0.0	Full	10	0.0	0.0
Approach	394	3.0	394	3.0		0.205		0.2	NA	0.0	0.0				
All Vehicles	848	3.0	848	3.0		0.205		1.8	NA	0.3	1.9				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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New Site Site Category: (None) Give-Way (Two-Way)

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use a	ane Use and Performance														
	Demand [ Total veh/h	Flows HV ] %	Arrival   [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	Aver. Bacl [ Veh	k Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1	374	3.0	374	3.0	1877	0.199	100	1.3	LOS A	0.0	0.0	Full	10	0.0	0.0
Approach	374	3.0	374	3.0		0.199		1.3	NA	0.0	0.0				
North: Harvey	/ Road														
Lane 1	363	3.0	363	3.0	1615	0.225	100	2.9	LOS A	0.4	2.7	Full	500	0.0	0.0
Approach	363	3.0	363	3.0		0.225		2.9	NA	0.4	2.7				
West: Conne	ctor Stree	et (west)													
Lane 1	146	3.0	146	3.0	810	0.181	100	8.2	LOS A	0.3	1.9	Full	500	0.0	0.0
Approach	146	3.0	146	3.0		0.181		8.2	LOS A	0.3	1.9				
All Vehicles	883	3.0	883	3.0		0.225		3.1	NA	0.4	2.7				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

## VSite: 101 [Ultimate\_Harvey-Connector east\_PM (Site Folder: General)]

New Site Site Category: (None) Give-Way (Two-Way)

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use a	nd Perfo	rmance	e												
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	Aver. Bac [ Veh	k Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1	411	3.0	411	3.0	1751	0.234	100	1.6	LOS A	0.3	1.9	Full	500	0.0	0.0
Approach	411	3.0	411	3.0		0.234		1.6	NA	0.3	1.9				
East: Connec	tor Street	(east)													
Lane 1	69	3.0	69	3.0	773	0.090	100	8.3	LOS A	0.1	0.9	Full	500	0.0	0.0
Approach	69	3.0	69	3.0		0.090		8.3	LOS A	0.1	0.9				
North: Harvey	y Road														
Lane 1	345	3.0	345	3.0	1916	0.180	100	0.4	LOS A	0.0	0.0	Full	10	0.0	0.0
Approach	345	3.0	345	3.0		0.180		0.4	NA	0.0	0.0				
All Vehicles	825	3.0	825	3.0		0.234		1.7	NA	0.3	1.9				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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#### ♥Site: 101 [Interim\_BS Rd-Harvey\_AM - 1,000 lots - SOUTH only (Site Folder: General)]

New Site Site Category: (None) Roundabout

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use ar	nd Perfor	mance	Э												
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Back [ Veh	Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1 d	713	3.0	713	3.0	946	0.753	100	16.9	LOS B	10.7	77.2	Full	500	0.0	0.0
Approach	713	3.0	713	3.0		0.753		16.9	LOS B	10.7	77.2				
East: Bannoc	kburn-She	elford R	load												
Lane 1 d	588	7.8	588	7.8	1441	0.408	100	4.4	LOS A	4.0	29.8	Full	500	0.0	0.0
Approach	588	7.8	588	7.8		0.408		4.4	LOS A	4.0	29.8				
West: Bannoo	kburn-Sh	elford F	Road												
Lane 1 d	546	9.4	546	9.4	654	0.835	100	23.9	LOS C	13.9	104.9	Full	500	0.0	0.0
Approach	546	9.4	546	9.4		0.835		23.9	LOS C	13.9	104.9				
All Vehicles	1847	6.4	1847	6.4		0.835		15.0	LOS B	13.9	104.9				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

d Dominant lane on roundabout approach

#### WSite: 101 [Interim\_BS Rd-Harvey\_PM - 1,000 lots - SOUTH only (Site Folder: General)]

New Site Site Category: (None) Roundabout

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use a	ne Use and Performance														
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Back [ Veh	Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1 d	295	3.0	295	3.0	809	0.364	100	11.9	LOS B	2.6	18.9	Full	500	0.0	0.0
Approach	295	3.0	295	3.0		0.364		11.9	LOS B	2.6	18.9				
East: Bannoc	kburn-She	elford R	oad												
Lane 1 d	1025	6.4	1025	6.4	1390	0.738	100	5.2	LOS A	11.0	81.3	Full	500	0.0	0.0
Approach	1025	6.4	1025	6.4		0.738		5.2	LOS A	11.0	81.3				
West: Banno	ckburn-Sh	elford F	Road												
Lane 1 d	494	8.8	494	8.8	1043	0.473	100	7.1	LOS A	4.0	29.9	Full	500	0.0	0.0
Approach	494	8.8	494	8.8		0.473		7.1	LOS A	4.0	29.9				
All Vehicles	1814	6.5	1814	6.5		0.738		6.8	LOS A	11.0	81.3				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

d Dominant lane on roundabout approach

New Site Site Category: (None) Roundabout

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use ar	nd Perfor	mance	)												
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Bacl [ Veh	k Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1 d	421	3.0	421	3.0	914	0.461	100	11.3	LOS B	3.5	24.9	Full	500	0.0	0.0
Approach	421	3.0	421	3.0		0.461		11.3	LOS B	3.5	24.9				
East: Bannoc	kburn-She	elford R	oad												
Lane 1 d	521	8.5	521	8.5	1274	0.409	100	4.9	LOS A	3.7	28.0	Full	500	0.0	0.0
Approach	521	8.5	521	8.5		0.409		4.9	LOS A	3.7	28.0				
West: Bannoo	ckburn-Sh	elford F	Road												
Lane 1 d	808	9.1	808	9.1	959	0.843	100	16.0	LOS B	16.4	123.4	Full	500	0.0	0.0
Approach	808	9.1	808	9.1		0.843		16.0	LOS B	16.4	123.4				
All Vehicles	1751	7.5	1751	7.5		0.843		11.6	LOS B	16.4	123.4				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

d Dominant lane on roundabout approach

New Site Site Category: (None) Roundabout

#### Site Layout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Lane Use ar	ne Use and Performance														
	Demand [ Total veh/h	Flows HV ] %	Arrival [ Total veh/h	Flows HV ] %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Back ( [ Veh	Of Queue Dist ] m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Harve	y Road														
Lane 1 d	237	3.0	237	3.0	699	0.339	100	12.2	LOS B	2.4	17.3	Full	500	0.0	0.0
Approach	237	3.0	237	3.0		0.339		12.2	LOS B	2.4	17.3				
East: Bannoc	kburn-She	elford R	oad												
Lane 1 d	954	7.8	954	7.8	1400	0.681	100	5.0	LOS A	8.9	66.4	Full	500	0.0	0.0
Approach	954	7.8	954	7.8		0.681		5.0	LOS A	8.9	66.4				
West: Bannoo	ckburn-Sh	elford F	Road												
Lane 1 d	520	9.0	520	9.0	1193	0.436	100	6.1	LOS A	3.8	28.7	Full	500	0.0	0.0
Approach	520	9.0	520	9.0		0.436		6.1	LOS A	3.8	28.7				
All Vehicles	1711	7.5	1711	7.5		0.681		6.3	LOS A	8.9	66.4				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

d Dominant lane on roundabout approach

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Appendix 3 – Concept plans



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Existing

Proposed ROW Boundary

---- Signalised Intersection Property Boundary

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WARNING

BEWARE OF UNDERGROUND SERVICES

HE LOCATIONS OF UNDERGROUND SERVICES ARE APPROXIMATE ONLY AND THEIR EXACT POSITION

## **Concept Plan**

DRAWING NO.

220912-CTP-02

ISSUE P2

2

SHEET NO.

20

RACV VICROADS MAP 93 D2 BANNOCKBURN SCALE OF METRES 10

PLANIT CONSULTING



Design by Trafficworks

---- Signalised Intersection Property Boundary

Proposed ROW Boundary

Cadastre

Existing

CLIENT

PLANIT CONSULTING

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## **Bannockburn Growth Plan** Golden Plains Shire Council

### **Concept Plan**

DRAWING NO.

RACV VICROADS MAP 93 D2 BANNOCKBURN SCALE OF METRES 10

SHEET NO.

3

20

220912-CTP-03



D	Drawing Record	Notes & Legend	TRAFFICWORKS		
ISSUE DRAWN APP'D DATE	AMENDMENT	1. AERIAL IMAGE FROM NEARMAP UNDER LICENSE AGREEMENT WITH			
P2 JF AA 24.01.23 PRE	LIMINARY ISSUE FOR COMMENT	TRAFFICWORKS PTY LTD.         2. ALL DIMENSIONS ARE TO FACE OF KERB UNLESS SHOWN OTHERWISE.			
WARNING BEWARE OF UNDERGROUND SI APPROXIMATE ONLY AND THEIR EXAC SHOULD BE PROVEN ON SITE. NO GU GIVEN THAT ALL EXISTING SERVICES	SERVICES ERVICES ARE CT POSITION ARANTEE IS ARE SHOWN,	Design by Trafficworks     Cadastre     Proposed ROW Boundary     Existing     Signalised Intersection Property Boundary	CLIENT PLANIT CONSULTING	RACV VICROADS MAP 93 D2 BANNOCKBURN SCALE OF METRES 0 5 10	SHEET NO.

## **Concept Plan**

DRAWING NO.

220912-CTP-04





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## **Bannockburn Growth Plan** Golden Plains Shire Council

## **Concept Plan**

DRAWING NO.

220912-CTP-05



## Appendix 4 – Swept path analysis





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## **Bannockburn Growth Plan** Golden Plains Shire Council

#### Swept Path Assessment





P2

WARNING



# Golden Plains Shire Council

## Swept Path Assessment



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- Design by Trafficworks
- Cadastre Proposed ROW Boundary

Existing

PLANIT CONSULTING

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P2

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# Golden Plains Shire Council

#### Swept Path Assessment

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220912-SKT-01-03



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# Golden Plains Shire Council

## Swept Path Assessment



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Design by Trafficworks Cadastre

Proposed ROW Boundary

Existing

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WARNING

BEWARE OF UNDERGROUND SERVICES

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# Golden Plains Shire Council

## Swept Path Assessment



DATE OF ISSUE: 24/01/23

SHEET NO. 10



Existing

DATE OF ISSUE: 24/01/23

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PLANIT CONSULTING



10

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			Drawing Record	Notes & Legend	TRAFFICWORKS		
D	AWN AP	PD DATE	AMENDMENT	1. AERIAL IMAGE FROM NEARMAP UNDER LICENSE AGREEMENT WITH			
	JF A	VARN OF UNDERG ONS OF UNDERG	23 PRELIMINARY ISSUE FOR COMMENT ING ROUND SERVICES BIOUND SERVICES ARE HER EXACT POSITION PRELIMINARY PLAN FOR DISCUSSION PURPOSES ONLY SUBJECT TO CHANGE WITHOUT NOTIFICATION	TRAFFICWORKS PTY LTD.     2. ALL DIMENSIONS ARE TO FACE OF KERB UNLESS SHOWN OTHERWISE.     Design by Trafficworks     Cadastre     Proposed ROW Boundary     Existing	CLIENT PLANIT CONSULTING	RACV VICROADS MAPS 93 D2 BONNOCKBURN SCALE DE METRES	

## Swept Path Assessment



Drawing Record	Notes & Legend	TRAFFICWORKS		
DRAWN         APP'D         DATE         AMENDMENT           JF         AA         24.01.23         PRELIMINARY ISSUE FOR COMMENT	1. AERIAL IMAGE FROM NEARMAP UNDER LICENSE AGREEMENT WITH TRAFFICWORKS PTY LTD.		(-w / È-)	
WARNING           BEWARE OF UNDERGROUND SERVICES           THE LOCATIONS OF UNDERGROUND SERVICES ARE APPROXIMATE ONLY AND THEIR EXACT POSITION SHOULD BE PROVEN ON SITE. NO GUARANTEE IS GIVEN THAT ALL EXISTING SERVICES ARE SHOWN.	2. ALL DIMENSIONS ARE TO FACE OF KERB UNLESS SHOWN OTHERWISE.      Design by Trafficworks     Cadastre     Proposed ROW Boundary     Existing	CLIENT PLANIT CONSULTING	RACV VICROADS MAPS 93 D2 BONNOCKBURN SCALE OF METRES 0 5 10	SHEET NO

## Swept Path Assessment



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## Swept Path Assessment

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			Drawing Record	Notes & Legend		
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## Swept Path Assessment

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				Drawing Record	Notes & Legend	TRAFFICWORKS	
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## **Bannockburn Growth Plan** Golden Plains Shire Council

## Swept Path Assessment

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			Drawing Record	Notes & Legend			
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		Drawing Record	Notes & Legend	<b>TRAFFICWORKS</b> <sup>™</sup>		
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## Bannockburn Growth Plan Golden Plains Shire Council

## Swept Path Assessment

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				Drawing Record	Notes & Legend	TRAFFICWORKS		
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## Swept Path Assessment

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220912-SKT-03-01



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	2.	ALL DIMENSIONS ARE TO FACE OF KERB UNLESS SHOWN OTHERWISE.

Design by Trafficworks

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Proposed ROW Boundary
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	2.	ALL DIMENSIONS ARE TO FACE OF KERB UNLESS SHOWN OTHERWISE.

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- Cadastre Proposed ROW Boundary

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## Bannockburn Growth Plan Golden Plains Shire Council

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